

GENERAL STRUCTURAL NOTES

PART I - DESIGN CRITERIA

A. GENERAL BUILDING CODE

1. THE CONSTRUCTION DOCUMENTS ARE BASED ON THE REQUIREMENTS OF THE FLORIDA BUILDING CODE SIXTH EDITION (2017).

B. WIND LOADS

1. WIND PRESSURES ARE BASED ON THE PROVISIONS OF THE FLORIDA BUILDING CODE WHICH REFER TO THE AMERICAN SOCIETY OF CIVIL ENGINEERS, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, ASCE 7-10 AND THE FOLLOWING CRITERIA:
 - a. ULTIMATE DESIGN WIND SPEED (VULT): 141 MPH (3 SECOND GUST)
 - b. NOMINAL DESIGN WIND SPEED (VASD): 110 MPH (3-SECOND GUST)
 - c. BUILDING RISK CATEGORY: II
 - d. WIND EXPOSURE CATEGORY: B
 - e. INTERNAL PRESSURE COEFFICIENT (GCPI): +0.55/-0.55

2. WIND PRESSURES USED FOR THE DESIGN OF COMPONENTS AND CLADDING ARE SHOWN IN THE FOLLOWING TABLE:

CLADDING TYPE	LOCATION OR ZONE	EFFECTIVE WIND AREA (SQ-FT)	WIND LOAD (PSF)
WALL	INTERIOR	10	+44/-47
WALL	INTERIOR	20	+43/-46
WALL	INTERIOR	50	+41/-44
WALL	INTERIOR	100	+40/-42
WALL	END ZONE	10	+44/-55
WALL	END ZONE	20	+43/-52
WALL	END ZONE	50	+41/-49
WALL	END ZONE	100	+40/-46
ROOF	INTERIOR	10	+26/-47
ROOF	INTERIOR	20	+25/-47
ROOF	INTERIOR	50	+24/-45
ROOF	INTERIOR	100	+23/-44
ROOF	EDGE	10	+26/-72
ROOF	EDGE	20	+25/-65
ROOF	EDGE	50	+24/-57
ROOF	EDGE	100	+23/-50
ROOF	CORNER	10	+26/-102
ROOF	CORNER	20	+25/-86
ROOF	CORNER	50	+24/-66
ROOF	CORNER	100	+23/-50

PART I - DESIGN CRITERIA (CONTINUED)

NOTES:

- a. WIDTH OF END ZONE/EDGE/CORNER STRIP: 3 FEET
- b. COMPONENT AND CLADDING PRESSURES ACT NORMAL TO THE SURFACE. POSITIVE PRESSURES ACT TOWARDS THE SURFACE AND NEGATIVE PRESSURES ACT AWAY FROM THE SURFACE.
- c. DESIGN PRESSURE FOR COMPONENTS AND CLADDING SHALL NOT BE LESS THAN 16 PSF ACTING IN EITHER DIRECTION NORMAL TO THE SURFACE.
- d. THE EFFECTIVE WIND AREA IS THE SPAN LENGTH MULTIPLIED BY AN EFFECTIVE WIDTH THAT NEED NOT BE LESS THAN ONE-THIRD THE SPAN LENGTH. FOR CLADDING FASTENERS, THE EFFECTIVE WIND AREA SHALL NOT BE GREATER THAN THE AREA THAT IS TRIBUTARY TO AN INDIVIDUAL FASTENER.
- e. THE DESIGN PRESSURES LISTED ABOVE ARE CALCULATED USING A VALUE OF KD OF 0.85. THE VALUES MUST BE INCREASED BY 18% UNLESS LOAD COMBINATIONS SPECIFIED IN ASCE 7-10 ARE USED IN DESIGN.

PART II - REINFORCED CONCRETE

A. CLASSES OF CONCRETE

1. ALL CONCRETE SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:


- a. ALL CONCRETE: 4,000 PSI NWC AT 28 DAYS, MAXIMUM AGGREGATE SIZE 1-1/2". CONCRETE SHALL BE PROPORTIONED TO COMPLY WITH ACI 318-314 TABLES 19.3.2.1, 19.3.3.1, AND 26.4.2.2b.

B. REINFORCING STEEL

1. ALL REINFORCING STEEL SHALL BE ASTM A 615 GRADE 60 UNLESS NOTED OTHERWISE ON THE DRAWINGS OR IN THESE NOTES.
2. REINFORCING STEEL: PROVIDE REINFORCING STEEL CONFORMING TO ASTM A 706 FOR ALL REINFORCING STEEL REQUIRED TO BE WELDED AND WHERE NOTED ON THE DRAWINGS.

C. REINFORCING STEEL COVERAGE

1. REINFORCING STEEL COVERAGE SHOULD CONFORM TO THE REQUIREMENTS SPECIFIED IN DETAILS. COVER IN STRUCTURAL MEMBERS NOT SPECIFIED IN THE DETAILS SHALL CONFORM TO THE REQUIREMENTS OF ACI 318 UNLESS SPECIFIED OTHERWISE ON THE DRAWINGS.

 walter p moore	Project Name: Rogers Park Golf Course Cart Barn 4101 Jim Walter Blvd. Tampa, Florida 33612	Dylan S. Richard P.E. NO. 60997 PROJECT NO. S05-19024-00	Date: 09/10/19	<h1 style="font-size: 2em;">S101</h1>
	Client: Tampa Sports Authority		Project Number: S05-19024-00	
813.221.2424 Certificate of Authorization No. 3818			Client 10136.00	

GENERAL STRUCTURAL NOTES

PART III - STRUCTURAL STEEL

A. MATERIAL

1. HOT ROLLED STRUCTURAL MEMBERS: ALL HOT ROLLED STEEL PLATES, SHAPES, SHEET PILING, AND BARS SHALL BE NEW STEEL CONFORMING TO ASTM SPECIFICATION A 6.
2. ASTM SPECIFICATION AND GRADE: CLEARLY MARK THE GRADE OF STEEL ON EACH PIECE, WITH A DISTINGUISHING MARK VISIBLE FROM FLOOR SURFACES, FOR THE PURPOSE OF FIELD INSPECTION OF PROPER GRADE OF STEEL. UNLESS NOTED OTHERWISE ON THE DRAWINGS, STRUCTURAL STEEL SHALL BE AS FOLLOWS:
 - a. BASE PLATES: ALL BASE PLATES SHALL CONFORM TO ASTM A 36 UNLESS NOTED OTHERWISE ON THE DRAWINGS.
 - b. OTHER STEEL: ANY OTHER STEEL NOT INDICATED OTHERWISE SHALL CONFORM TO ASTM A 992 OR ASTM A 572, GRADE 50, EXCEPT PLATES AND ANGLES THAT SHALL BE ASTM A 36.

B. STRUCTURAL BOLTS AND THREADED FASTENERS

1. A 325 BOLTS: ALL BOLTS IN STRUCTURAL CONNECTIONS SHALL CONFORM TO ASTM A 325 TYPE 1, UNLESS INDICATED OTHERWISE ON THE DRAWINGS.

C. WELDING

1. UNLESS NOTED OTHERWISE, ELECTRODES FOR WELDING SHALL CONFORM TO E70XX (SMAW), F7XX-EXXX (SAW), ER70S-X (GMAW), OR E7XT-X (FCAW).
2. ELECTRODES FOR GRADE 60 OR GRADE 65 MATERIAL SHALL CONFORM TO E80XX (SMAW), F8XX-EXX-XX (SAW), ER80S-X (GMAW), OR E8XT-X (FCAW).

D. ANCHOR RODS

1. UNLESS INDICATED OTHERWISE IN THE COLUMN SCHEDULE OR ON THE DRAWINGS, ANCHOR RODS SHALL CONFORM TO ASTM F 1554 GRADE 55 (WITH SUPPLEMENTARY REQUIREMENT S1) AND THE SIZE SHALL BE 3/4" DIAMETER AND SHALL EMBED INTO THE CONCRETE FOUNDATION A DISTANCE OF 1'-0" WITH A HEAVY HEX NUT AT THE EMBEDDED END. STRIKE BOLT THREADS AT THE EMBEDDED END AT TWO PLACES BELOW THE NUT.

E. GROUT

1. GROUT BELOW STRUCTURAL STEEL BASE PLATES SHALL BE NON-METALLIC, NON-SHRINK GROUT WITH A MINIMUM STRENGTH OF 6,000 PSI WHEN BEARING ON 3,000 PSI CONCRETE OR LESS, A STRENGTH OF 8,000 PSI WHEN BEARING ON CONCRETE BETWEEN 3,000 AND 4,000 PSI, AND, UNLESS NOTED OTHERWISE ON THE DRAWINGS, A STRENGTH OF 8,000 PSI WHEN BEARING ON CONCRETE GREATER THAN 4,000 PSI.

PART IV - SUBMITTALS

A. THE GENERAL CONTRACTOR SHALL SUBMIT FOR ENGINEER REVIEW SHOP DRAWINGS FOR THE FOLLOWING ITEMS:

1. CONCRETE MIX DESIGNS.
2. REINFORCING STEEL.
3. STRUCTURAL STEEL, SHOP AND ERECTION DRAWINGS.


PART V - MISCELLANEOUS

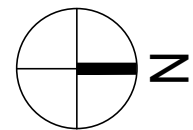
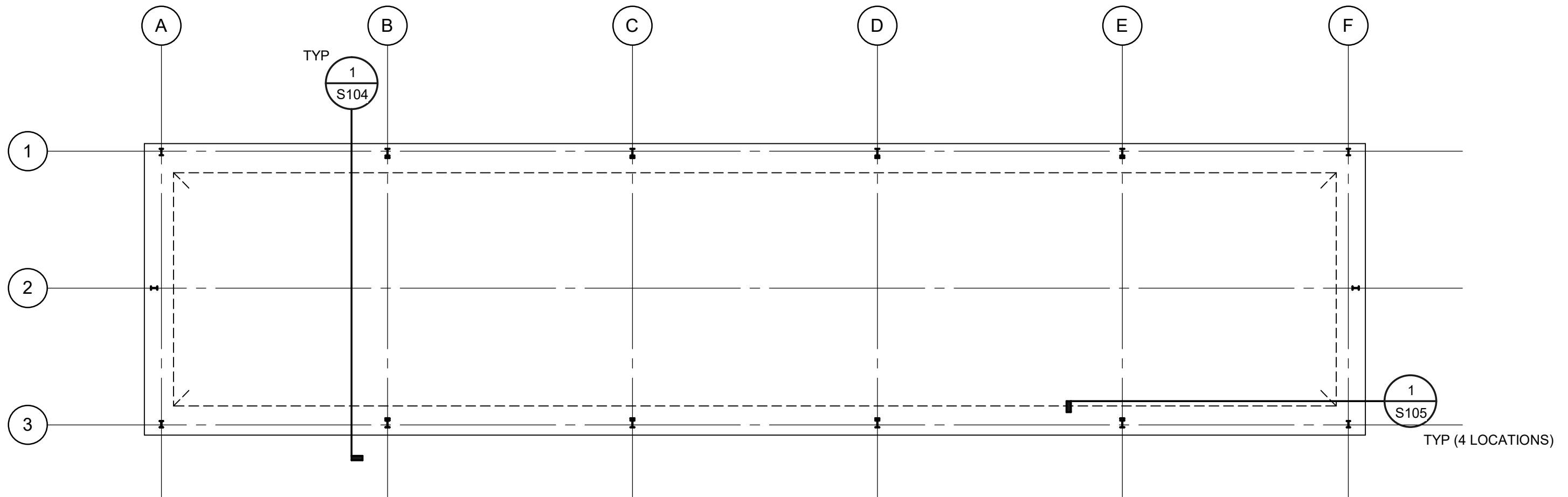
A. THE STRUCTURAL ENGINEER'S ROLE DURING CONSTRUCTION

1. THE ENGINEER SHALL NOT HAVE CONTROL NOR CHARGE OF, AND SHALL NOT BE RESPONSIBLE FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, OR PROCEDURES, FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK, FOR THE ACTS OR OMISSION OF THE CONTRACTOR, SUBCONTRACTOR, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS

B. MAINTENANCE STATEMENT

1. ALL STRUCTURES REQUIRE PERIODIC MAINTENANCE TO EXTEND LIFESPAN AND TO ENSURE STRUCTURAL INTEGRITY FROM EXPOSURE TO THE ENVIRONMENT. A PLANNED PROGRAM OF MAINTENANCE SHALL BE ESTABLISHED BY THE BUILDING OWNER. THIS PROGRAM SHALL INCLUDE SUCH ITEMS SUCH AS BUT NOT LIMITED TO PAINTING OF STRUCTURAL STEEL, PROTECTIVE COATING FOR CONCRETE, SEALANTS, CAULKED JOINTS, EXPANSION JOINTS, CONTROL JOINTS, SPALLS AND CRACKS IN CONCRETE, AND PRESSURE WASHING OF EXPOSED STRUCTURAL ELEMENTS EXPOSED TO A SALT ENVIRONMENT OR OTHER HARSH CHEMICALS.

 walter p moore	Project Name: Rogers Park Golf Course Cart Barn 4101 Jim Walter Blvd. Tampa, Florida 33612	Dylan S. Richard P.E. NO. 60997 PROJECT NO. S05-19024-00	Date: 09/10/19	Sheet: <h1 style="font-size: 2em;">S102</h1>
	Client: Tampa Sports Authority		Project Number: S05-19024-00	
Walter P Moore and Associates, Inc. 201 East Kennedy Blvd, Suite 300 Tampa, Florida 33602 813.221.2424 Certificate of Authorization No. 3818				




GOLF COURSE CART BARN PLAN VIEW

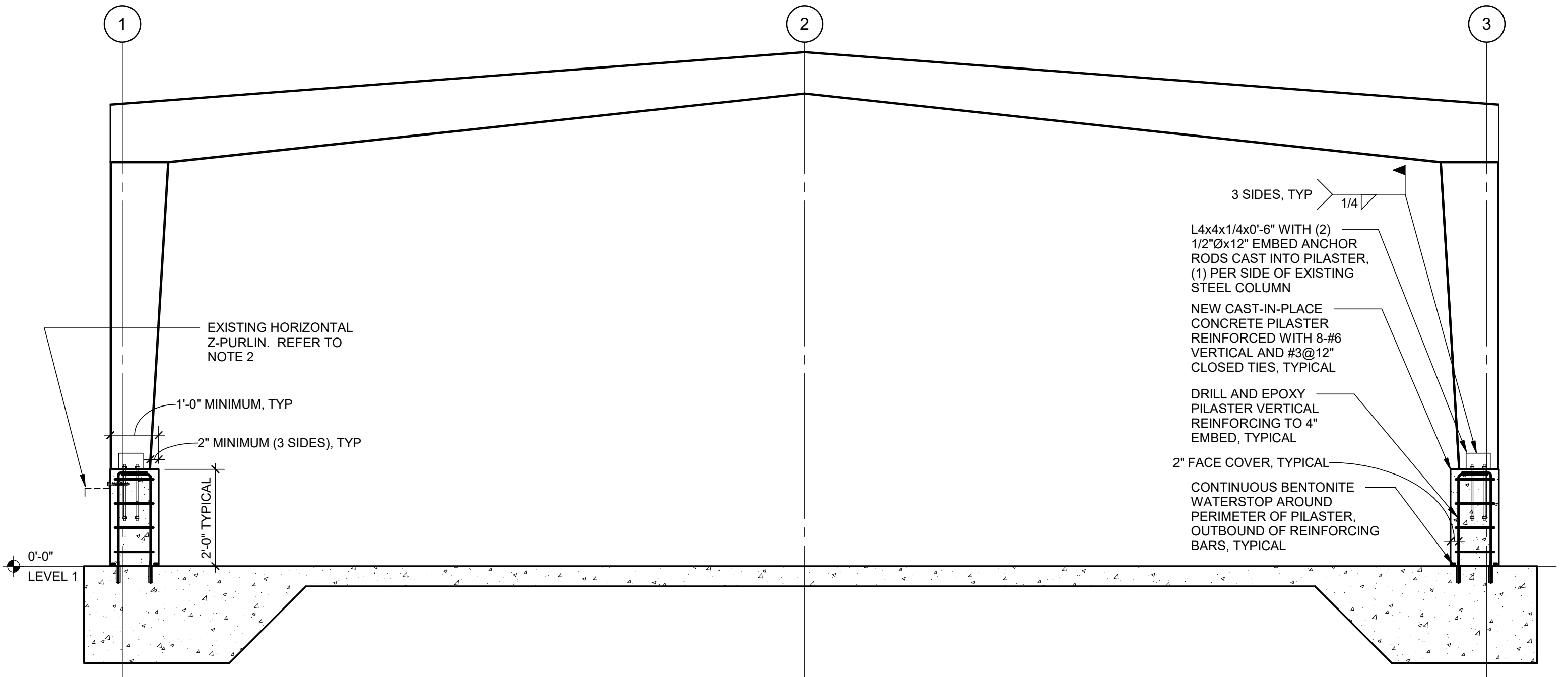
1

3/32" = 1'-0"

NOTES:

1. REFER TO PHASE 1 DRAWINGS S001, S002, AND S003 FOR SHORING REQUIREMENTS.
2. SEQUENCE FOR REMEDIATION PER DETAILS SHALL BE AS FOLLOWS: ONLY (3) COLUMN LOCATIONS SHALL BE WORKED ON AT ANY GIVEN TIME, AND ALL (3) SHALL BE ADJACENT TO ONE ANOTHER. FOR EXAMPLE, PROPOSED SEQUENCE IS AS FOLLOWS:
 - COLUMNS A, B, C ALONG GRID 3
 - COLUMNS D, E, F ALONG GRID 3
 - COLUMNS A, B, C ALONG GRID 1
 - COLUMNS D, E, F ALONG GRID 1
 - COLUMNS A, F ALONG GRID 2
 CONTRACTOR SHALL SUBMIT PROPOSED SEQUENCE FOR REVIEW AND APPROVAL PRIOR TO ANY REMEDIATION WORK.
3. ADD ALTERNATE 1:
 - ALL STEEL SHALL BE CLEANED OF SURFACE RUST VIA SAND BLAST AND SHALL BE COATED WITH CORROSIVE RESISTANT PAINT OR SIMILAR PROTECTION.
4. ADD ALTERNATE 2:
 - LOWEST HORIZONTAL Z-PURLINS ARE SHOWING SIGNIFICANT SIGNS OF DETERIORATION AND ARE RECOMMENDED TO BE REPLACED WITH SIZES TO MATCH EXISTING.

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	Dylan S. Richard P.E. NO. 60997 PROJECT NO. S05-19024-00	Engineer: DSR Drafter: JLM	
		Client: 10136.00	



3 SIDES, TYP $\frac{1}{4}$

L4x4x1/4x0'-6" WITH (2) 1/2"Øx12" EMBED ANCHOR RODS CAST INTO PILASTER, (1) PER SIDE OF EXISTING STEEL COLUMN

NEW CAST-IN-PLACE CONCRETE PILASTER REINFORCED WITH 8-#6 VERTICAL AND #3@12" CLOSED TIES, TYPICAL

DRILL AND EPOXY PILASTER VERTICAL REINFORCING TO 4" EMBED, TYPICAL

2" FACE COVER, TYPICAL

CONTINUOUS BENTONITE WATERSTOP AROUND PERIMETER OF PILASTER, OUTBOUND OF REINFORCING BARS, TYPICAL

EXISTING HORIZONTAL Z-PURLIN. REFER TO NOTE 2

1'-0" MINIMUM, TYP

2" MINIMUM (3 SIDES), TYP

2'-0" TYPICAL

0'-0"
LEVEL 1


NOTES:

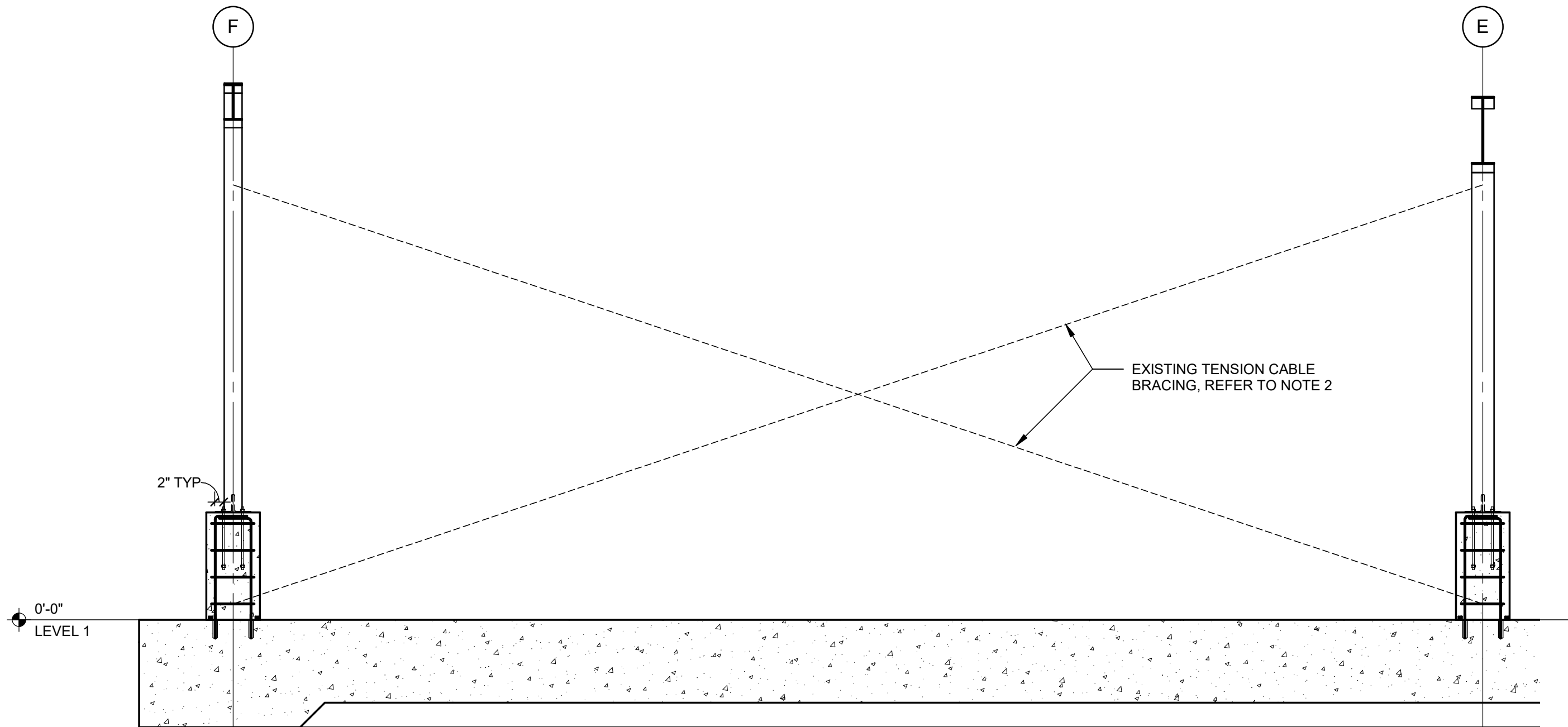
- EXISTING CORRODED COLUMNS TO BE CUT FLUSH AT LOCATION SHOWN.
- IF EXISTING HORIZONTAL Z-PURLIN CONNECTION TO EXISTING STEEL COLUMN IS INTERRUPTED DUE TO LOCATION OF NEW CAST-IN-PLACE CONCRETE PILASTER, PROVIDE NEW CONNECTION OF EXISTING Z-PURLIN TO PILASTER. ANCHOR TO PILASTER USING 1/2"Øx4" EMBED HILTI KWUK HUZ-EZ SCREW ANCHOR AT ENDS. PROVIDE MINIMUM 3" EDGE AND TOP DISTANCE TO ANCHOR.

GOLF COURSE CART BARN COLUMN REMEDIATION

1

1/2" = 1'-0"

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	<p>Client:</p> <p style="text-align: center;">Tampa Sports Authority</p>	<p>Project Number:</p> <p>S05-19024-00</p>	
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NOTES:

1. REFER TO OTHER DETAILS FOR BALANCE OF INFORMATION NOT SHOWN.
2. EXISTING TENSION CABLE BRACING BOTTOM CONNECTIONS TO BE RELOCATED 1'-0" ABOVE TOP OF NEW CAST-IN-PLACE CONCRETE PILASTER. ONLY ONE TENSION CABLE BRACE SHALL BE RELOCATED AT A TIME, AND SHALL BE FULLY RECONNECTED AND TIGHTENED PRIOR TO RELOCATION OF ANY OTHER BRACING. NEW CONNECTION TO MATCH EXISTING CONNECTIONS, FIELD VERIFY.

GOLF COURSE CART BARN BRACE REMEDIATION

1

1/2" = 1'-0"

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